

William J. Clinton Presidential Center

Little Rock, AR

Blake Haney Emmanuel Hernandez Andrea Baiardi

Content

- Introduction
- Background
- Gravity System
- Lateral System
- Loads
- Superstructure Design
- Foundation

Introduction

PROJECT NAME: William J. Clinton Presidential Center

LOCATION: Little Rock, Arkansas

COST: \$160 million

ARCHITECT: Polshek Partnership Architects, LLP, NYC

STRUCT'L: Leslie E. Robertson Associates, RLLP, NYC

PROJECT DESIGN

BEGAN:

1999

CONSTRUCTION

BEGAN:

2002

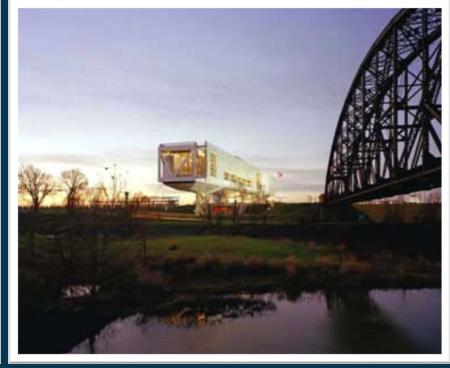
DATE OPENED: July 28, 2004

Background

- Building serves as a library and museum
- President Clinton wanted to convey openness and accessibility
- Architects wanted the building to resemble the six nearby bridges
- Mostly made out of glass and structural steel





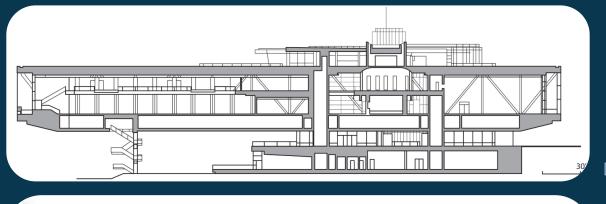


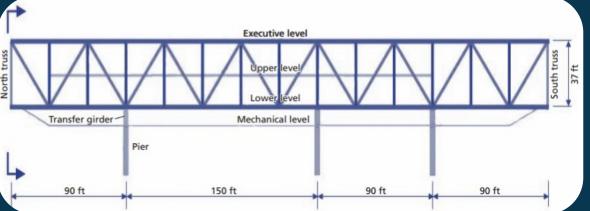
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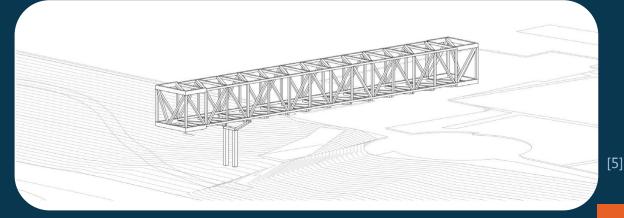


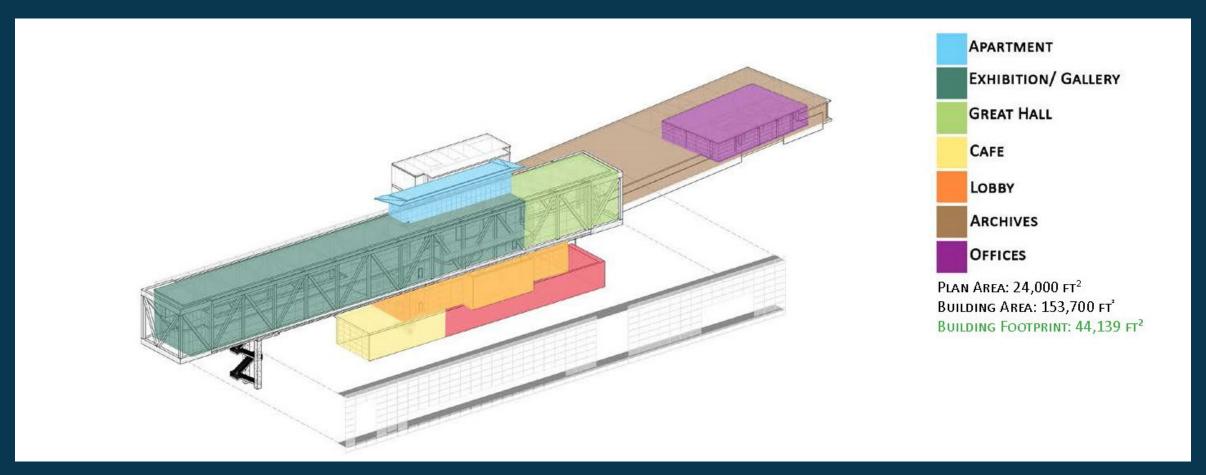
Dimensions

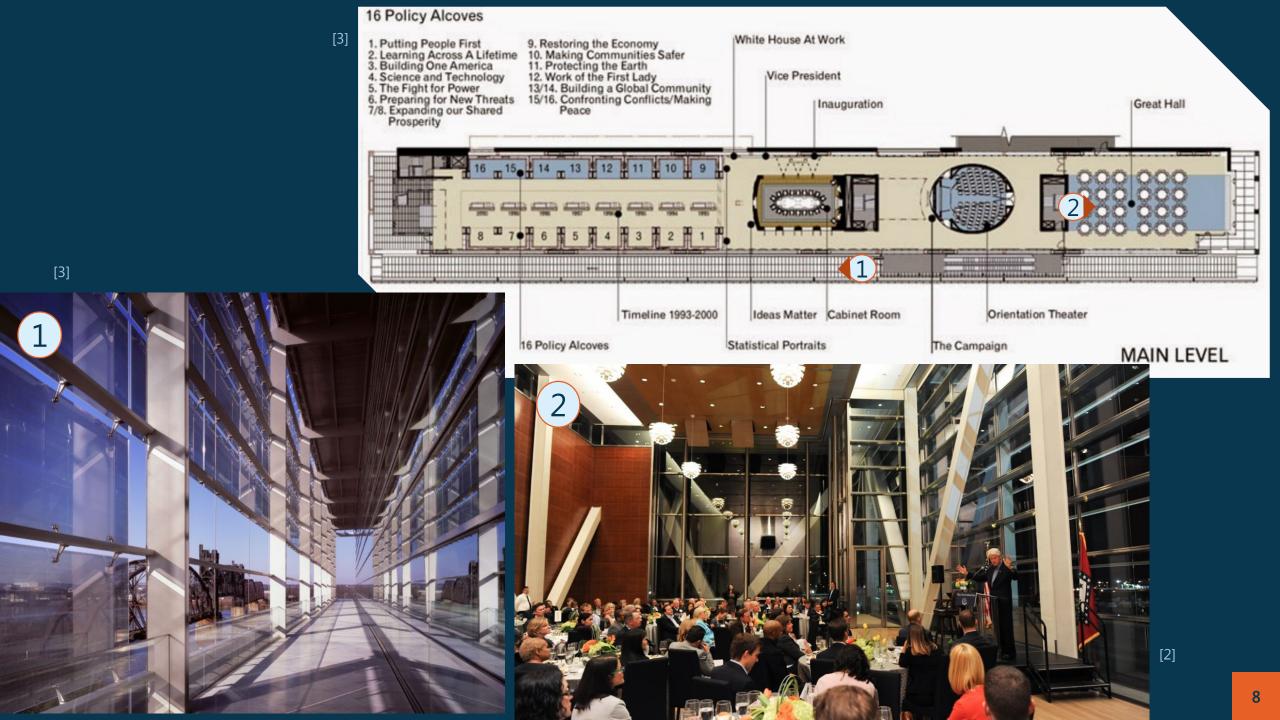
- Five-story structure
- Main 420 ft. long body of structure supported by 37 ft. deep trusses that cantilever 90 ft. towards river
- Trusses spaced 46 ft. apart
- 165,000 sf

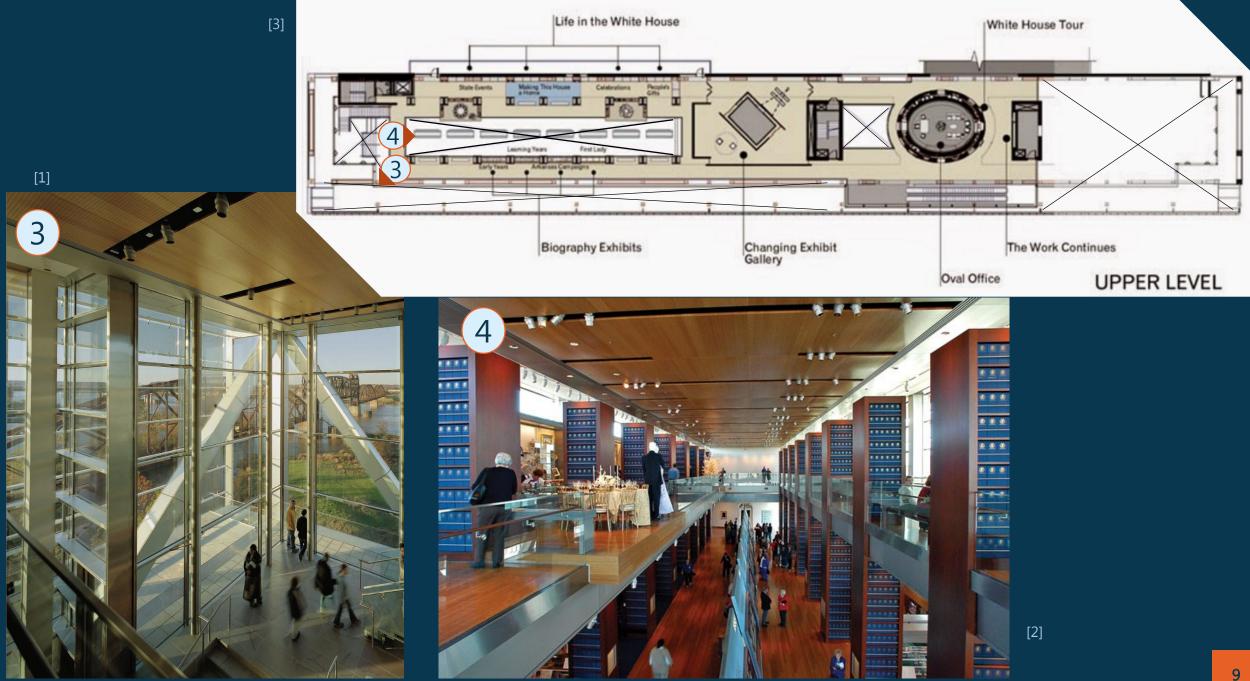






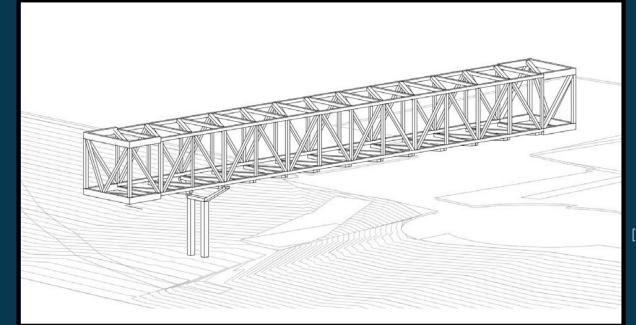


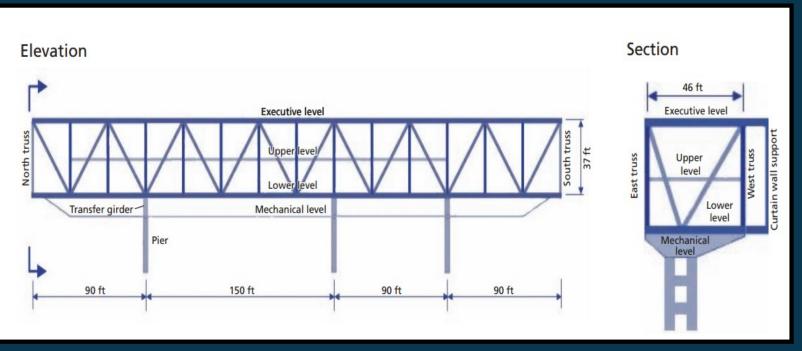




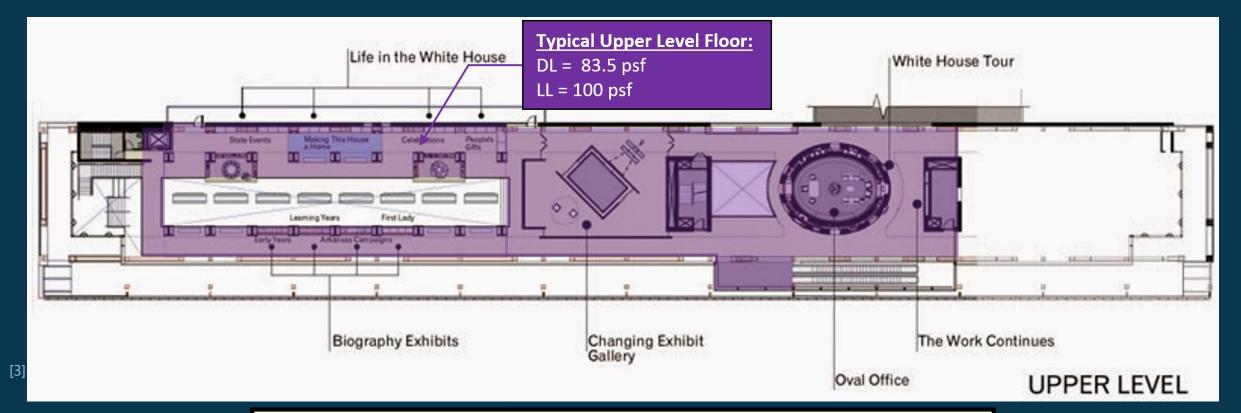
Gravity System

- Pair of deep trusses make up the structural frame in the 420 ft. direction
 - Supported at three locations
 - Cantilevers 90 ft. on both ends
- Horizontal trusses span the width at the top and bottom chords of the deep truss
 - Bolted in the field to top flanges and floor beams

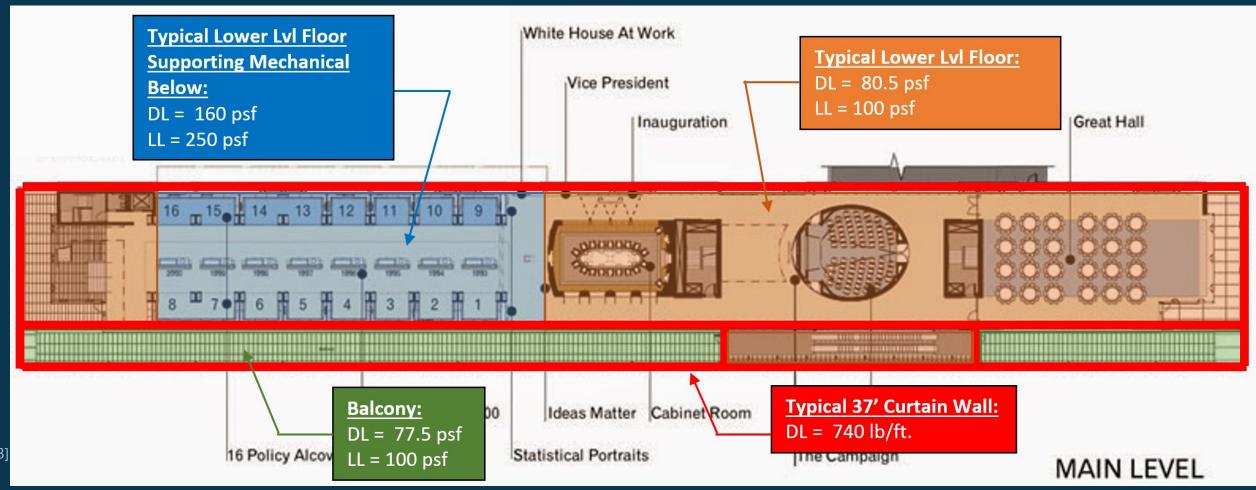




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Typical Upper Level Floor			
5" Conc. Slab, Normal Weight	67.5	psf	
Deck	2	psf	
Flooring	3	psf	
Ceiling	3	psf	
Floor Collateral	5	psf	
Floor Sprinklers	3	psf	
Total Dead Load	83.5	psf	
Live Load	100	psf	(reducible)



Typical Lower Level Floor			
5" Conc. Slab, Normal Weight	67.5	psf	
Deck	2	psf	
Flooring	3	psf	
Floor Collateral	5	psf	
Floor Sprinklers	3	psf	
Total Dead Load	80.5	psf	
Live Load	100	psf	(reducible)

Typical Mechanical Level Floor			
5" Conc. Slab, Normal Weight	67.5	psf	
Deck	2	psf	
Flooring	3	psf	
Insulation		psf	
Soffit	5	psf	
Total Dead Load	79.5	psf	
Live Load	150	psf	(reducible)

77.5	psf	
5	psf	
3	psf	
2	psf	
67.5	psf	
	2 3 5	3 psf

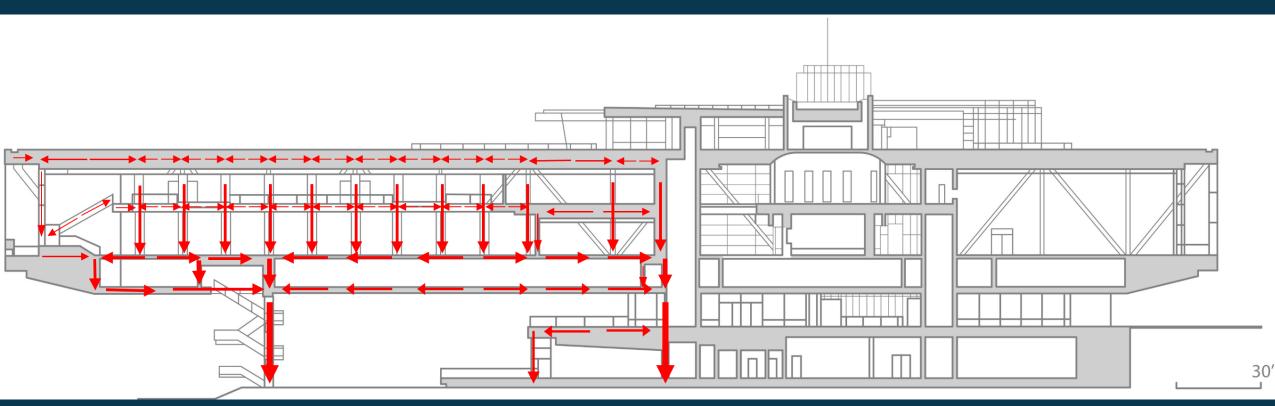
Other Loading Plans Not Shown

Typical Penthouse Roof			
Roofing	10	psf	
Roof Deck	2	psf	
Insulation	2	psf	
Ceiling	3	psf	
Roof Collateral	5	psf	
Sprinklers	3	psf	
Total Dead Load	25	psf	
Live Load	20	psf	(unreducible)

Typical Roof Garden			
Soil, 1 ft. thick	120	psf	
5" Conc. Slab, Normal Weight	67.5	psf	
Deck	2	psf	
Insulation	2	psf	
Ceiling	3	psf	
Roof Collateral	5	psf	
Sprinklers	3	psf	
Total Dead Load	202.5	psf	
Live Load (1 ft. thick soil)	120	psf	(unreducible)

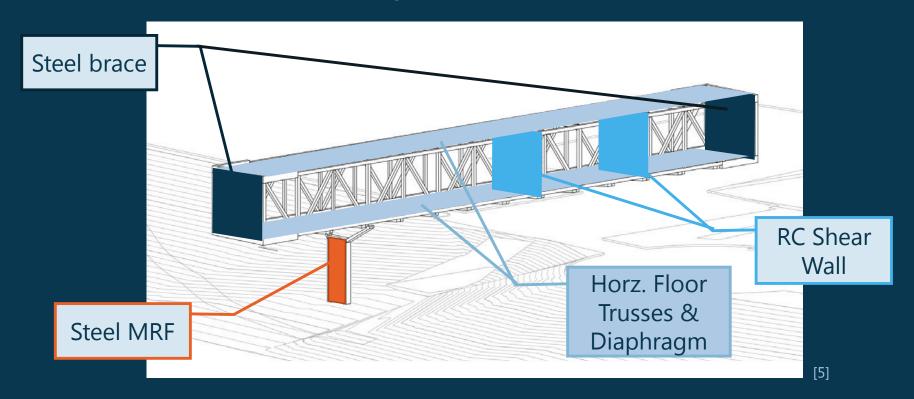
Typical Executive Level Floor			
5" Conc. Slab, Normal Weight	67.5	psf	
Deck	2	psf	
Flooring	3	psf	
Ceiling	3	psf	
Floor Collateral		psf	
Floor Sprinklers	3	psf	
Total Dead Load	83.5	psf	
Live Load	100	psf	(reducible)

Gravity Load Tracing



Lateral System

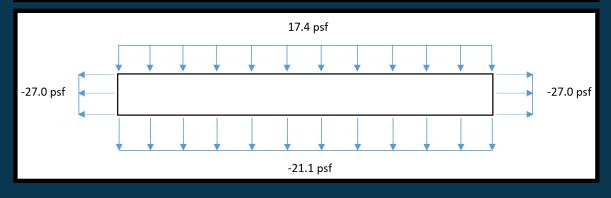
- Shear walls, horizontal floor trusses and reinforced concrete floor diaphragms make up the lateral load resisting system
 - Shear walls located at outer perimeter of the building's stair towers
- Floor diaphragms help distribute the lateral load
- Horizontal trusses act like drag member and collectors, transferring the lateral loads to shear walls
 - Deliver lateral wind and seismic forces to the building's shear walls

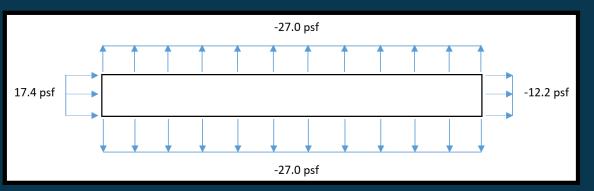


Wind Loads	
Basic Wind Speed	115 mph
Wind Exposure Category	C
Wind Importance Factor, I	1.0
Internal Pressure Coefficient, C_{pi}	+/- 0.18
Gust Effect Factor, G	0.85
Mean Roof Height, h	74 ft.
Wind Directionality Factor, K_d	0.85
Topographic Factor, K_{zt}	1.0
Velocity Pressure Exposure Coefficient, K_h	1.21 (at mean roof height)

Wind Load in the Short Direction (L/B = 0.11)				
Wall	c_p	q_hGC_p	$q_h GC_{pi}$	
		(psf)	(psf)	
Windward	0.80	23.7	±6.26	
Leeward	-0.50	-14.8	±6.26	
Side	-0.70	-20.7	±6.26	

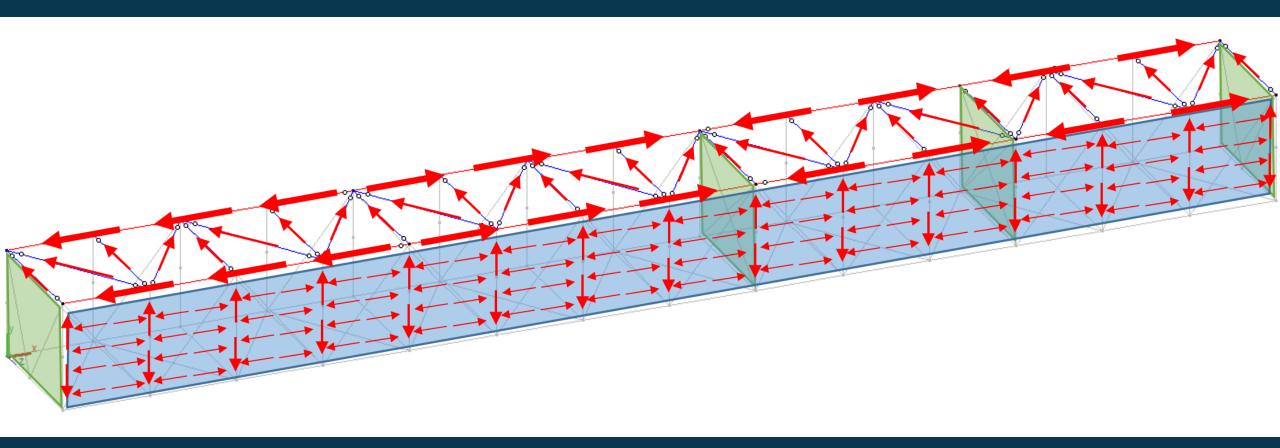
1	Wind Load in the Long Direction (L/B = 9.1)				
Wall	c_p	q_hGC_p	$q_h GC_{pi}$		
		(psf)	(psf)		
Windward	0.80	23.7	±6.26		
Leeward	-0.20	-5.92	±6.26		
Side	-0.70	-20.7	±6.26		





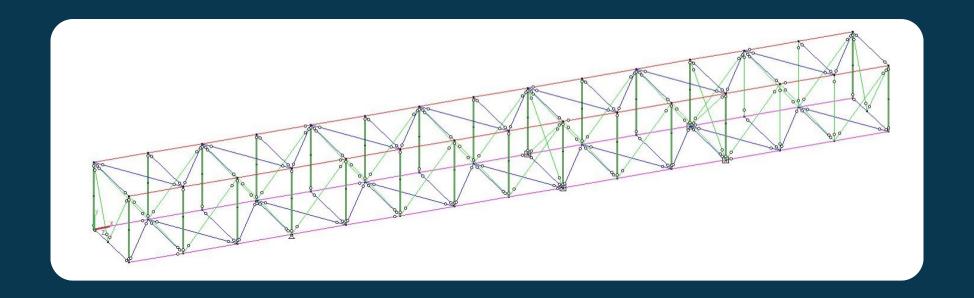
Seismic Loads Spectral Response Coeff., S_{DS} Spectral Response Coeff., S_{D1} Site Class Seismic Design Category (Short Period) Seismic Design Category (Long Period) Approximate Period Parameter C_t Approximate Period Parameter x Structural Height, h_n Upper Limit Coeff., C_u	0.399 0.234 D (assumed) C D 0.02 0.75 74 ft. 1.466
Approx. Period Upper Limit, C_uT_a Fundamental Period (minimum), T Period T_0 Short Period T_S Long Period T_L	0.740 s 0.505 s 0.117 s 0.586 s 12 s
Analysis Procedure Response Modification Coeff., R Overstrength Factor, Ω_0 Deflection Amplification Factor, C_d Importance Factor, I_e Seismic Response Coeff., C_s Seismic Weight, W	Equivalent Lateral Force Method (ASCE 7-10, Section 12.8) 5 2.5 4.5 1.0 0.0798 10,000 k

Wind Load Tracing



Structural Analysis

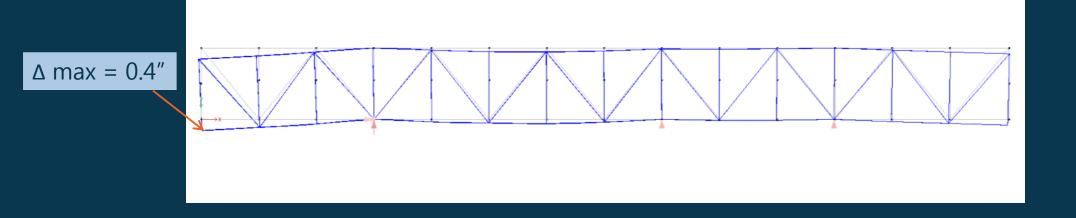
- Supports modeled as pin and rollers, restrained for out of plane translations at the base
- Moment releases on member ends, identifying locations of simple connections
- Shear walls modeled with two X-braced frames
- Top and bottom chords modeled as continuous, with the proper splice locations



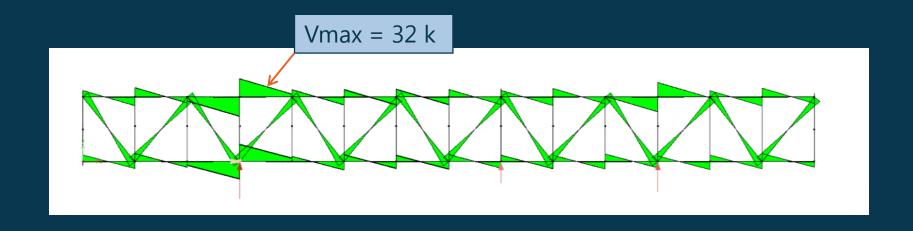
Structural Analysis

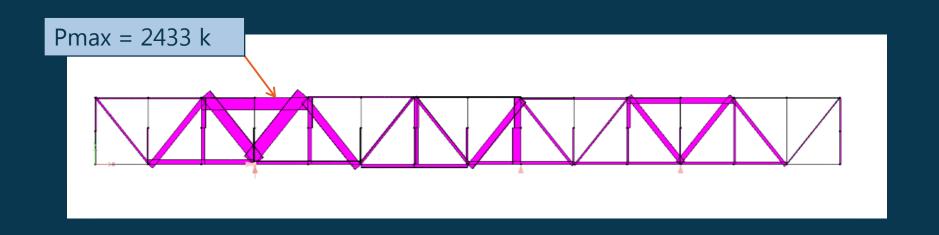
- Performed the structural analysis in Multiframe
- LRFD load combinations considered:
 - 1.4D
 - 1.2D+1.6L+0.5Lr
 - 1.2D+1.6Lr+L
 - 1.2D+1.6Lr+0.5W
 - 1.2D+1.0W+L+0.5Lr
 - 1.2D+1.0L
 - 0.9D+1.0W

- Typical Dead Loads
 - Self-weight
 - P~35-100k
- Typical Live Loads
 - P~27-150k
- Typical Wind Loads
 - P~11-22k

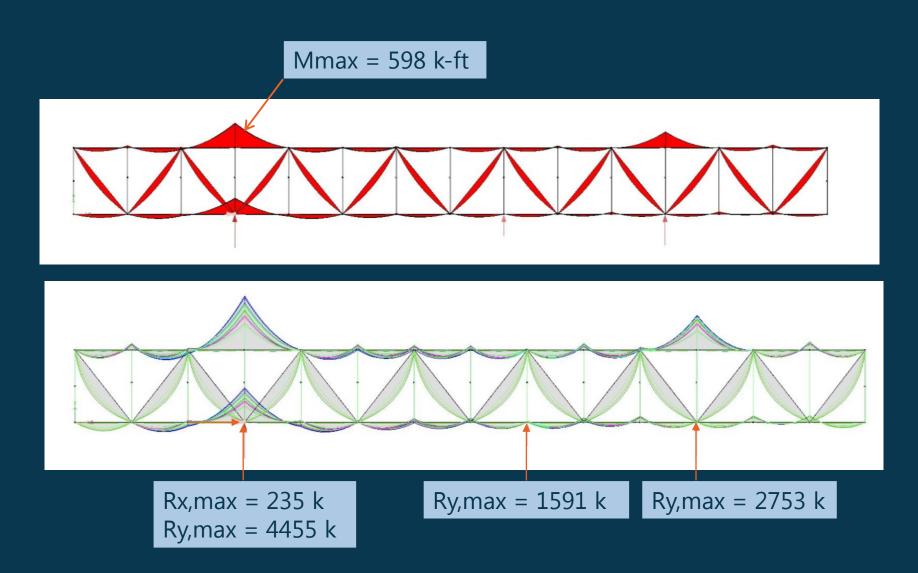


Structural Analysis (Gravity)

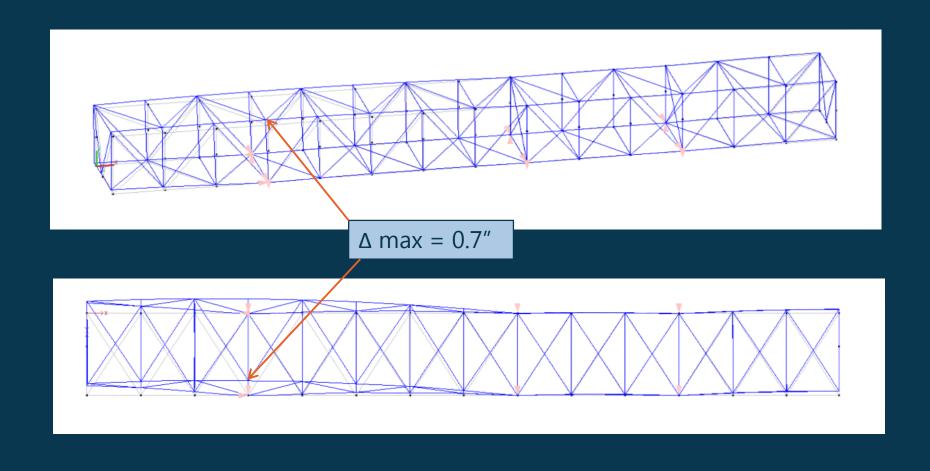




Structural Analysis



Structural Analysis (Lateral)



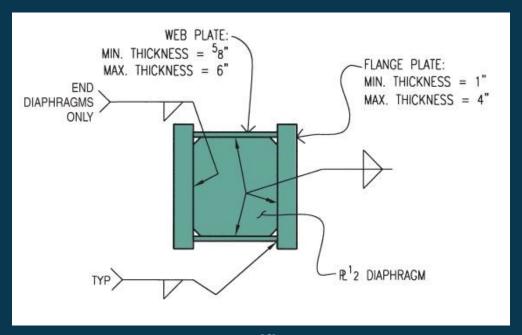
Structural Design

Member	Section
Bottom chords of trusses	Built up steel boxes 26"D x 16" W
Architecturally exposed diagonals	Built up steel boxes 16"D x 16" W

- Chord and diagonals have same width due to architectural concerns, but also provided a simple family of connection details, practical for construction
- Built up members made out of ASTM A572 GR50 steel

Structural Design

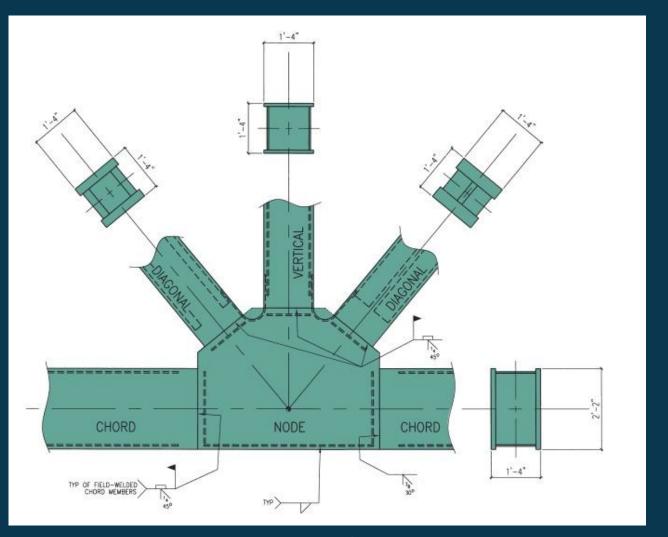
- Truss consists of:
 - 30 vertical members
 - 32 diagonal members
 - 36 chord members
 - 36 nodes
- Truss members are built-up box sections
 - Ranging in length form 28 to 125 ft.
 - Required very high standards of dimensional control
 - Required full length fillet welds



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Structural Design

 Truss was connected on site with groove welds

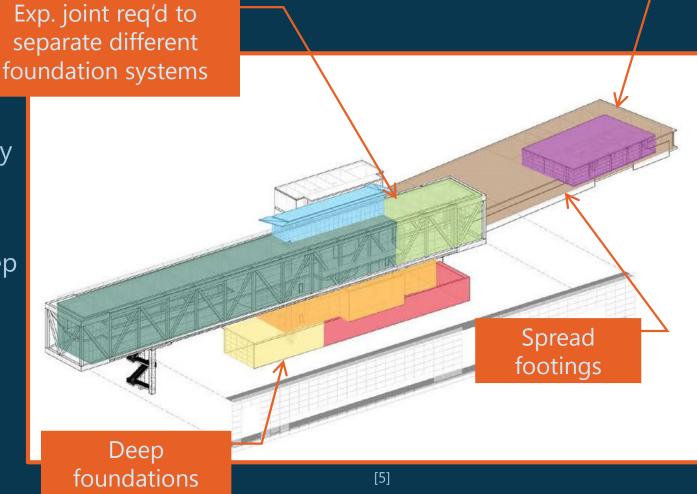


Foundations

Archive bldg.

 Expansive clay was undercut and backfilled with engineered fill

- The foundation for the museum/library was separated from the archive building with expansion joint
- Museum/library building contains deep foundation design per the recommendation of the geotechnical engineers
- Archive building contains concrete spread footings



Foundation Design

- Several 60 to 80 ft. long drilled shaft caissons
- Diameters ranged from 30 to 42 in.
- Embedded 20 ft. into bedrock shale
- Designed to resist overturning forces and to deliver lateral loads to the bedrock

Sources

- [1] D. A. Sesil and O. Güleç, "Commanding Presence," Civil Engineering Magazine, vol. 75, no. 3, pp. 42-49, March 2005.
- [2] The Clinton Foundation, "The Clinton Presidential Center," [Online]. Available: https://www.clintonfoundation.org/clinton-presidential-center. [Accessed 28 March 2017].
- [3] J. Hill, "A Daily Dose of Architecture: Clinton Library," 21 February 2005. [Online]. Available: http://archidose.blogspot.com/2005/02/clinton-library.html. [Accessed 28 March 2017].
- [4] Garcia, "Structural Precedent Study," [Online]. Available: http://www.arch.ttu.edu/courses/2013/fall/3501/Students/Garcia/04/Default.htm. [Accessed 28 March 2017].
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- [6] C. Rautenberg, "Making the Essential Connections on "A Bridge to the 21st Century"," *Welding Innovation,* vol. XX, no. 1, 2003.
- [7] Hargreaves Associates, "William J. Clinton Presidential Center," [Online]. Available: http://www.hargreaves.com/projects/Institutional/WJClintonCenter/. [Accessed 19 April 2017].