beams: bending and shear stress
Galileo
- relationship between stress and depth²

- can see
  - top squishing
  - bottom stretching

what are the stress across the section?
Pure Bending

- bending only
- no shear
- axial normal stresses from bending can be found in
  - homogeneous materials
  - plane of symmetry
  - follow Hooke’s law
Bending Moments

- **sign convention:**

- **size of maximum internal moment will govern our design of the section**
Normal Stresses

- geometric fit
  - plane sections remain plane
  - stress varies linearly
Neutral Axis

- stresses vary linearly
- zero stress occurs at the centroid
- neutral axis is line of centroids (n.a.)
Derivation of Stress from Strain

• pure bending = arc shape

\[ L = R \theta \]

\[ L_{outside} = (R + y)\theta \]

\[ \varepsilon = \frac{\delta}{L} = \frac{L_{outside} - L}{L} = \frac{(R + y)\theta - R \theta}{R \theta} = \frac{y}{R} \]
Derivation of Stress

- zero stress at n.a.

\[ f = E \varepsilon = \frac{E_y}{R} \]

\[ f_{\text{max}} = \frac{E_c}{R} \]

\[ f = \frac{y}{c} f_{\text{max}} \]
Bending Moment

- resultant moment from stresses = bending moment!

\[ M = \Sigma f_y \Delta A \]

\[ = \Sigma \frac{y f_{\text{max}}}{c} y \Delta A = \frac{f_{\text{max}}}{c} \Sigma y^2 \Delta A = \frac{f_{\text{max}}}{c} I = f_{\text{max}} S \]
# Bending Stress Relations

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Transverse Loading and Shear

- perpendicular loading
- internal shear
- along with bending moment
Bending vs. Shear in Design

- **bending stresses dominate**

- **shear stresses exist horizontally with shear**

- **no shear stresses with pure bending**
Shear Stresses

• horizontal & vertical
Shear Stresses

- horizontal & vertical
Beam Stresses

- horizontal with bending

![Diagram of beam stresses](image)
Equilibrium

- **horizontal force** $V$ needed

\[ V_{\text{longitudinal}} = \frac{V_T Q}{I} \Delta x \]

- **$Q$ is a moment area**
Moment of Area

- \( Q \) is a moment area with respect to the n.a. of area above or below the horizontal

- \( Q_{\text{max}} \) at \( y=0 \) (neutral axis)

- \( q \) is shear flow:
  \[
  q = \frac{V_{\text{longitudinal}}}{\Delta x} = \frac{V_T Q}{I}
  \]
Shearing Stresses

\[ f_v = \frac{V}{DA} = \frac{V}{b \cdot \Delta x} \]

\[ f_{v-\text{ave}} = \frac{VQ}{Ib} \]

- \( f_v = 0 \) on the top/bottom
- \( b \text{ min} \) may not be with \( Q \text{ max} \)
- with \( h/4 \geq b, f_{v-\text{max}} \leq 1.008 f_{v-\text{ave}} \)
Rectangular Sections

\[ I = \frac{bh^3}{12} \quad Q = A\bar{y} = \frac{bh^2}{8} \]

\[ f_v = \frac{VQ}{Ib} = \frac{3V}{2A} \]

- \( f_{v-max} \) occurs at n.a.
Steel Beam Webs

- **W and S sections**
  - $b$ varies
  - stress in flange negligible
  - presume constant stress in web

\[
\sigma_{v\text{-max}} = \frac{3V}{2A} \approx \frac{V}{A_{\text{web}}}
\]
Shear Flow

- loads applied in plane of symmetry
- cut made perpendicular

\[ q = \frac{VQ}{I} \]
Shear Flow Quantity

• sketch from $Q$

\[ q = \frac{VQ}{I} \]
Connectors Resisting Shear

- plates with
  - nails
  - rivets
  - bolts
- splices

\[
\frac{V_{\text{longitudinal}}}{p} = \frac{VQ}{I}
\]

\[
nF_{\text{connector}} \geq \frac{VQ_{\text{connected area}}}{I} \cdot p
\]
Vertical Connectors

- isolate an area with vertical interfaces

\[ nF_{\text{connector}} \geq \frac{VQ_{\text{connected area}}}{I} \cdot p \]
Unsymmetrical Shear or Section

- member can bend and twist
  - not symmetric
  - shear not in that plane
- shear center
  - moments balance