

**ARCH 631. Final Exam Reference Formulas**  
from Lectures, Examples, and Assignments

|  |   |   |
|--|---|---|
| $\sum F_x = R_x = 0$   | $\sum F_y = R_y = 0$  | $\sum M = 0$  |
| $F_x = F \cos \theta$  | $F_y = F \sin \theta$   | $M = Fd$  |
| $F = \sqrt{F_x^2 + F_y^2}$   | $\tan \theta = \frac{F_y}{F_x}$   | $\tan = \frac{\text{opposite}}{\text{adjacent}}$  |
| $\cos = \frac{\text{adjacent}}{\text{hypotenuse}}$   | $\sin = \frac{\text{opposite}}{\text{hypotenuse}}$  | $\delta_T = \alpha(\Delta T)L$  |
| $R_{(\text{case 1})} = \frac{wl}{2}$   | $f = E\varepsilon$  | $\varepsilon = \frac{\delta}{L}$  |
| $\delta = \frac{PL}{AE}$   | $W = wl$  | $w = w' \times \text{tributary width}$  |
| $V = \int -w dx$   | $M = \int V dx$   | area of a rectangle = $b \times h$  |
| area of a triangle = $\frac{b \times h}{2}$  | area of a trapezoid = $b \times \frac{(h_1 + h_2)}{2}$  | $x = \frac{V_A}{w}$   |
| $M_{\text{max}(\text{case 1})} = \frac{wl^2}{8}$   | $f_{b-\text{max}} = \frac{M}{S} \leq F_b$   | $f_{v-\text{max}} = \frac{3V}{2A} \leq F_v$ for a rectangle   |
| $V_{\text{max}(\text{case 1})} = \frac{wl}{2}$   | $S_{\text{req}} \geq \frac{M}{F_b}$ $S_{\text{req}'d} \geq \frac{M}{F_b}$   | $A_{\text{req}'d} \geq \frac{3V}{2F_v}$ for a rectangle   |
| $\Delta_{\text{max}(\text{case 1})} = \frac{5wl^4}{384EI}$   | $\Delta_{\text{max}} \leq \Delta_{\text{limit}}$  | $\Delta_{\text{limit}(\text{LL or TL})}$ in form of $L/\text{number}$   |
| 1.4D<br>1.2D + 1.6L + 0.5(L <sub>r</sub> or S or R)<br>1.2D + 1.6(L <sub>r</sub> or S or R) +<br>(L or 0.5W)<br>1.2D + 1.0W + L +<br>0.5(L <sub>r</sub> or S or R)<br>1.2D + 1.0E + L + 0.2S<br>0.9D + 1.0W<br>0.9D + 1.0E | $I_x = \frac{bh^3}{12}$ for a rectangle<br><br>$I_y = \frac{b^3h}{12}$ for a rectangle<br><br>$P_{cr} = \frac{\pi^2 EI}{(L_e)^2} = \frac{\pi^2 EA}{\left(\frac{L_e}{r}\right)^2}$ | $T_v = \frac{wl}{2}$<br><br>$T_h = \left(\frac{wL}{2} \cdot \frac{L}{4}\right) \frac{1}{h}$<br><br>$T = \sqrt{T_v^2 + T_h^2}$ |
| $w = \gamma A$   | $f_c = \frac{P}{A} \leq F'_c$   | $I_x = \sum I_x + \sum A \cdot d_y^2$   |
| $W = mg$   | $S = \frac{I}{c}$   | $I_y = \sum I_y + \sum A \cdot d_x^2$   |
| $g = 9.81 \text{ m/s}^2$   | $L_e = KL$  | $L_r = L_o \left(0.25 + \frac{15}{\sqrt{K_{LL} A_T}}\right)$  |
| $R_y = \frac{wL}{2}$   | $R_x = \left(\frac{wL}{2} \cdot \frac{L}{4}\right) \frac{1}{h}$   | $R = \sqrt{R_x^2 + R_y^2}$  |

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| $f_t = \frac{P}{A}$   | $f_v = \frac{P}{A}$                           | $f_p = \frac{R}{A}$   |
| $f_v = \frac{VQ}{Ib}$   | $Q_y = \bar{x}A = \sum_{i=1}^n \bar{x}_i A_i$ | $nF_{connector} \geq \frac{VQ_{connected\ area}}{I} \cdot p$        |
| $s = x \left[ 1 + \frac{2}{3} \left( \frac{y}{x} \right)^2 + \dots \right]$ | $y = 4h(Lx - x^2) / L^2$                      | $L_{total} = L(1 + \frac{3}{8} h^2 / L^2 - \frac{32}{5} h^4 / L^4)$ |

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| $\sum F_x = 0$   | $\sum F_y = 0$  | $\sum M = 0$   |
| $V = \int -wdx$  | $M = \int Vdx$  | $M = Fd$   |
| area of a rectangle = $b \times h$   | area of a triangle = $\frac{b \times h}{2}$   | area of a trapezoid = $b \times \frac{(h_1 + h_2)}{2}$   |
| fixed base portal frame with assumed inflection points and lateral load:<br>$R_{1x} = R_{2x} = P/2$<br>$R_{1y} = -0.45 Ph/L$<br>$R_{2y} = 0.45 Ph/L$<br>$M_1 = M_2 = 0.275 Ph$<br>$M_{joints} = 0.225 Ph$  | $f_{max} = \frac{P}{A} + \frac{Mc}{I} = \frac{P}{A} + \frac{M}{S}$<br>$w = \gamma A$<br>simply supported at corners square plate<br>$m_{max} = 0.15w'a^2$<br>$2. 1.2D + 1.6L$ | three span beam with integral end supports:<br>$V_{max} = 1.15 wL/2$<br>$M_{max+ends} = wL^2/14$<br>$M_{max+interior} = wL^2/16$<br>$M_{max-column} = wL^2/16$<br>$M_{max-ends, other\ support} = wL^2/10$<br>$M_{max-interior} = wL^2/11$ |
| $M = Td$   | slab depth = $L/\text{number}$ or $\ell_n/\text{number}$  |  |
| $\ell_n = \ell - \frac{1}{2} \text{ support 1 width} - \frac{1}{2} \text{ support 2 width}$  | $m = M/\text{strip width}$  | $M_o = (M_T) = \frac{w_u \ell_2 \ell_n^2}{8}$  |
| flat slab, interior bay, COLUMN strip:<br>$M_{negative} = (0.75 \cdot 0.65 M_T) = 0.49 M_o$<br>$M_{positive} = (0.60 \cdot 0.35 M_T) = 0.21 M_o$<br>flat slab, interior bay, MIDDLE strip:<br>$M_{negative} = (0.25 \cdot 0.65 M_T) = 0.16 M_o$<br>$M_{positive} = (0.40 \cdot 0.35 M_T) = 0.14 M_o$ | fully fixed square plate<br>$m_{negative} = 0.0513 w' a^2$<br>$a = \frac{A_s f_y}{0.85 f'_c b}$<br>$M_{max(case 1)} = \frac{wl^2}{8}$<br>$\rho = \frac{A_s}{bd}$              | $\phi M_n = \phi A_s f_y (d - a/2) \quad \phi = 0.9 *$<br>to determine maximum steel for 0.005 strain<br>$c = 0.375d, a = \beta_1 c$<br>$R_n = \frac{M_n}{bd^2}$ or $\frac{M_u}{\phi bd^2}$<br>$V_{x(case 1)} = wl/2 - wx$                 |
| beam: $A_{s-min} = \frac{3\sqrt{f'_c}}{f_y} (b_w d)$ not less than $\frac{200}{f_y} (b_w d)$   |   | $\phi V_c = \phi 2\lambda \sqrt{f'_c} b_w d \quad \lambda = 1.0$ normal weight   |
| $\phi V_c \geq V_u > \frac{\phi V_c}{2} : s_{required} = \text{smaller of } \frac{A_v f_{yt}}{50b_w}$ and $\frac{A_v f_{yt}}{0.75\sqrt{f'_c} b_w}$   |   | $V_u \leq \phi V_c + \phi V_s \quad \phi = 0.75$   |
| $V_u > \phi V_c : s_{required} = \frac{\phi A_v f_{yt} d}{V_u - \phi V_c}$ or $\frac{\phi A_v f_{yt} d}{\phi V_s}$   | $P = f \cdot A$   | sphere: $T = p_r R/2$  |
| $N_\phi = \frac{Rw}{1 + \cos \phi} = \frac{W}{2\pi R \sin^2 \phi}$   | $T = N_\phi \cos \phi a$  | cylinder: $T = p_r R$  |

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| $N_\theta = R w \left( -\frac{1}{1 + \cos \phi} + \cos \phi \right)$ | $a = R \sin \phi$   | $w = \gamma t$  | $f = T/t$               |
| single overhang at 0.292L:<br>$M = 0.043 w L^2$                      | double overhangs at 0.21L:<br>$M = 0.021 w L^2$   | $f_\phi = N_\phi/t$   | $f_\theta = N_\theta/t$ |
| $P_o = 0.85 f'_c (A_g - A_{st}) + f_y A_{st}$                        | $L_{LL} = L_o \left( 0.25 + \frac{15}{\sqrt{K_{LL} A_T}} \right)$                         | slabs: $A_{s-min} = 0.002 b t$ (40 or 50 grade)<br>$A_{s-min} = 0.0018 b t$ (60 grade)                                  |                         |
| tiered: $\phi_c P_n = \phi_c (0.8 P_o) \phi_c = 0.65$                | $V_{u2} = w_u (\text{tributary area} - b_1 \times b_2)$                                   | $V_{u2} \leq \phi \left( 2 + \frac{4}{\beta_c} \right) \lambda \sqrt{f'_c} b_o d \leq \phi 4 \lambda \sqrt{f'_c} b_o d$ |                         |
| spiral: $\phi_c P_n = \phi_c (0.85 P_o) \phi_c = 0.75$               | $b_E = \min \left( \frac{\ell_n}{4}, b_w + 16t, \text{ or center-center spacing} \right)$ | $b_o = 2(b_1) + 2(b_2)$   |                         |
| $E_c = 57,000 \sqrt{f'_c}$ normal weight                             |   |   |                         |

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| $w = w' \times \text{tributary width}$                | $W = w \times \text{tributary height}$                             | $R = \frac{W}{\# \text{parallel support walls}}$     |
| $M = F d$   | $V = ZICW/R_w$   | $C = \frac{1.25S}{T^{2/3}}$                          |
| $M_{\max(\text{simply supported})} = \frac{w l^2}{8}$ | $f_v = \frac{P}{A}$  | $A_{\text{circle}} = \pi r^2 = \pi D^2/4$            |
| $f_v = \frac{P}{L(0.707t)}$                           | $f_v = \frac{P}{2A}$   | $R_u \leq \phi R_n$                                  |
| shear: $R_u \leq \phi r_n n$                          | bearing: $R_u \leq \phi r_n t n$                                   | capacity = min { all $\phi R_n$ 's }                 |
| $f_p = \frac{P}{t d}$                                 | $S = \frac{b h^2}{6}$  | $f_{b-max} = \frac{M}{S} \leq F_b$                   |
| $M_{\max(\text{cantilever})} = \frac{w l^2}{2}$       | $M_{\max(3 \text{ spans})} = 0.1 w l^2$                            | $V_{\max(\text{simply supported})} = \frac{w l}{2}$  |
| $v_{\text{wall}} = \frac{V}{L_{\text{wall}}}$         | $v_{\text{diaphragm}} = \frac{V}{L_{\text{in diaphragm}}}$         | $V = C_s W$  |
| $S_{MS} = F_d S_s$                                    | $C_s = \frac{S_{DS}}{(R/I)}$ not less than $\frac{S_{D1}}{T(R/I)}$ | $S_{DS} = \frac{2}{3} S_{MS}$                        |
| $P \leq n \cdot q$                                    | $f_v = \frac{VQ}{Ib}$  | $nF \geq \frac{VQ_{\text{connected}}}{I} p$          |
| $f_v = \frac{3V}{2A}$                                 | $\phi R_n = \phi F_u A_e \quad \phi = 0.75$                        | $A_e = A_n U$  |
| $\phi R_n = \phi S L \quad \phi = 0.75$               | $\phi R_n = \phi F_y A_g \quad \phi = 0.9$                         | $F' = C_D C_M C_F \dots \times F_{\text{tabulated}}$ |

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| $R_u \leq \phi(0.6F_u A_{nv} + U_{bs} F_u A_{nt} \leq 0.6F_y A_{gv} + U_{bs} F_u A_{nt}) \quad \phi = 0.75$ |  | $F_{cE} = \frac{0.822E'_{\min}}{\left(l_e/d\right)^2}$ |
| $w = \gamma A$  | $\Delta_{max} \leq \Delta_{limit}$                     | $E'_{\min} = E_{\min}(C_M)(C_t)(C_T)(C_i)$             |
| $S_{req} \geq \frac{M}{F'_b}$   | $\Delta_{max(simple supported)} = \frac{5wl^4}{384EI}$ | $F_c^* \cong F_c C_D$                                  |
| $\left[\frac{f_c}{F'_c}\right]^2 + \frac{f_{bx}}{F'_{bx}\left[1 - \frac{f_c}{F_{cEx}}\right]} \leq 1.0$     | $SF = \frac{M_{resist}}{M_{overturning}} \geq 1.5$     | $F'_c = F_c^* C_p$                                     |
|   |  | $P_a = F'_c A$   |

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| $M_{max(simple supported)} = \frac{wl^2}{8}$   | $\Delta_{max} \leq \Delta_{limit}$   | $M_u \leq \phi M_n$ or $M_a \leq \frac{M_n}{\Omega}$                              |
| 1.4D<br>1.2D + 1.6L + 0.5(L <sub>r</sub> or S or R)<br>1.2D + 1.6(L <sub>r</sub> or S or R) + (L or 0.5W)<br>1.2D + 1.0W + L + 0.5(L <sub>r</sub> or S or R)<br>1.2D + 1.0E + L + 0.2S<br>0.9D + 1.0W<br>0.9D + 1.0E | $P_u \leq \phi P_n$ or $P_a \leq \frac{P_n}{\Omega}$   | $r = \sqrt{\frac{db^3}{12(bd)}} = \frac{b}{\sqrt{12}}$                            |
|  | $I_{req'd} = \frac{(\Delta_{formula} \cdot I)}{\Delta_{limit value}}$  | $f_v = \frac{V}{A_{nv}} \leq F_v$   |
| $w = w' \times tributary width$  | $F_{vm} = \frac{1}{2} \left[ \left( 4 - 1.75 \left( \frac{M}{Vd} \right) \right) \sqrt{f'_m} \right] + 0.25 \frac{P}{A_n}$<br>where M/Vd is positive and cannot exceed 1.0 | $F_{vs} = 0.5 \left( \frac{A_v F_s d}{A_{nv} s} \right)$                          |
| $w_{adjusted} = w_{ll-have} \left( \frac{L/360}{L/400} \right)_{table/need}$   | $F_{v-max} = 3\sqrt{f'_m}$ for $M/Vd \leq 0.25$  | $F_v = F_{vm} + F_{vs}$   |
| $KL_x = \frac{KL_y}{(r_x/r_y)}$  | $F_{v-max} = 2\sqrt{f'_m}$ for $M/Vd \geq 1.0$   | $A_{nv} = bh - A_s$   |
| $P_a = [0.25f'_m A_n + 0.65A_{st} F_s] \left[ 1 - \left( \frac{h}{140r} \right)^2 \right]$ for h/r ≤ 99 (reinforced)   |  | $\frac{P}{A} \leq q_a$  |
| $P_a = [0.25f'_m A_n + 0.65A_{st} F_s] \left( \frac{70r}{h} \right)^2$ for h/r > 99 (reinforced)   |  | $q_u = \frac{P_u}{A}$   |
| $V_{u1} = BL'q_u$  | $\phi V_{n1} = \phi 2\lambda \sqrt{f'_c} B d \quad \phi = 0.75$  | $L_m = \frac{B}{2} - \frac{\bullet}{2}$<br>where • is smaller dimension of column |
| $V_{u2} = P_u - q_u(c+d)(b+d)$   | $b_o = 2(c+d) + 2(b+d)$  | $M_u = q_u \frac{BL_m^2}{2}$  |
| $\phi V_{n2} = \phi \left( 2 + \frac{4}{\beta} \right) \lambda \sqrt{f'_c} b_o d \leq \phi 4\lambda \sqrt{f'_c} b_o d \quad \phi = 0.75$   | $P = \frac{\gamma h^2}{2}$ at h/3  | $P_{e1} = \frac{\pi^2 EI}{(L_{c1})^2}$  |

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| $\frac{P_r}{P_c} \geq 0.2: \frac{P_r}{P_c} + \frac{8}{9} \left( \frac{M_{rx}}{M_{cx}} + \frac{M_{ry}}{M_{cy}} \right) \leq 1.0$               | $B_1 = \frac{C_m}{1 - \alpha(P_u/P_{e1})} \geq 1.0$           |   |
| $\frac{P_r}{P_c} < 0.2: \frac{P_r}{2P_c} + \left( \frac{M_{rx}}{M_{cx}} + \frac{M_{ry}}{M_{cy}} \right) \leq 1.0$                             | $C_m = 0.6 - 0.4 \frac{M_1}{M_2} \text{ where } M_1 < M_2$    |   |
| $I_{req'd} \geq \frac{\Delta_{toobig}}{\Delta_{limit}} I_{trial}$   | $V_u \leq \phi V_n = \phi(0.6 F_{yw}) t_w d \quad \phi = 1.0$ | $M_r = B_1 M_{nt}$  |
| $R_u \leq \phi R_n$   | bearing: $R_u \leq \phi r_n t n$                              | shear: $R_u \leq \phi r_n n$  |
| $\phi R_n = \phi(0.6) F_y A_g \quad \phi = 1.0$   | $\phi R_n = \phi(0.6) F_u A_{nv} \quad \phi = 0.75$           | $U_{bs} = 0.5$<br>when tensile stress is non-uniform  |
| $R_u \leq \phi(0.6 F_u A_{nv} + U_{bs} F_u A_{nt} \leq 0.6 F_y A_{gv} + U_{bs} F_u A_{nt}) \quad \phi = 0.75$                                 |   | $l \text{ is the largest of:}$<br>$m = \frac{N - 0.95d}{2}$<br>$n = \frac{B - 0.8b_f}{2}$                                   |
| $X = \frac{4db_f}{(d+b_f)^2} \cdot \frac{P_u}{\phi_c P_p} = \frac{4db_f}{(d+b_f)^2} \cdot \frac{P_u}{\phi_c(0.85f'_c)BN} \quad \phi_c = 0.65$ |   | $\lambda n' \text{ where } n' = \frac{\sqrt{db_f}}{4} \text{ and}$<br>$\lambda = \frac{2\sqrt{X}}{(1 + \sqrt{1-X})} \leq 1$ |
| $P_p = 0.85 f'_c A$   | $t_{min} = l \sqrt{\frac{2P_u}{0.9F_y BN}}$                   |   |
| $f_b - f_a \leq F_t$  | $f_b = \frac{Mc}{I} = \frac{M}{S}$                            | $M = Pe$  |
| $f_a + f_b \leq F_b$  | $M \leq M_s = A_s F_s (jd) \quad j = 0.909$                   | $\frac{f_a}{F_a} + \frac{f_b}{F_b} \leq 1$  |
| $f_a \leq F_a$  | $F_v = 1.5\sqrt{f'_m} \leq 120 \text{ psi unreinforced}$      | $f_v = \frac{VQ}{I_n b} = \frac{3V}{2A} \text{ solid rectangle}$  |
| with $M = Vh, M/Vd = h/d$   | $F_b = 0.45 f'_m \text{ reinforced}$                          | $F_b = 0.33 f'_m \text{ unreinforced}$  |
| $F = \mu N$   | $SF = \frac{M_{resist}}{M_{overturning}} \geq 1.5 - 2$        | $SF = \frac{F_{horizontal-resist}}{F_{sliding}} \geq 1.25 - 2$  |